

Engineering data and specifications

Design data – metal framing channel

Table 1

Elements of sections properties for design

Nominal thickness (inches)

12 ga = 0.105

14 ga = 0.075

16 ga = 0.060

Legend

I Moment of inertia

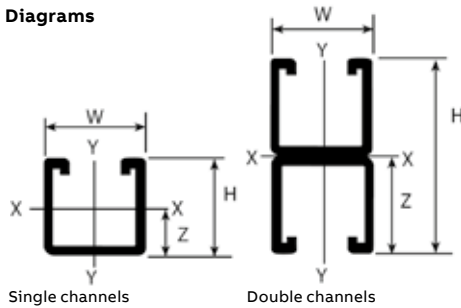
S Section Modulus

r Radius of Gyration

Z Nominal Axis

A Area

Diagrams



Single channels

Double channels

Section member	Wt. lb/ft.	H (in.)	W (in.)	A (in.) ²	I (in.) ⁴	S (in.) ³	X-X axis			Y-Y axis	
							r (in.)	Z (in.)	I (in.) ⁴	S (in.) ³	r (in.)
Single channel											
A1200	1.90	1.625	1.625	0.557	0.192	0.212	0.587	0.719	0.237	0.292	0.652
B1200	1.28	0.813	1.625	0.381	0.031	0.063	0.283	0.331	0.137	0.168	0.600
C1200	1.70	1.375	1.625	0.500	0.121	0.155	0.492	0.595	0.205	0.252	0.640
D1200	1.44	1.000	1.625	0.424	0.053	0.092	0.356	0.403	0.159	0.196	0.616
E1200	2.47	2.438	1.625	0.726	0.529	0.399	0.853	1.112	0.335	0.413	0.679
H1200	3.05	3.250	1.625	0.897	1.100	0.635	1.107	1.507	0.436	0.536	0.697
A1400	1.40	1.625	1.625	0.401	0.134	0.146	0.577	0.707	0.184	0.226	0.677
B1400	0.97	0.813	1.625	0.280	0.024	0.051	0.295	0.338	0.103	0.127	0.607
Double channel											
A1202	3.80	3.250	1.625	1.114	0.948	0.583	0.992	1.625	0.474	0.584	0.652
B1202	2.56	1.626	1.625	0.762	0.147	0.181	0.439	0.813	0.274	0.337	0.600
C1202	3.40	2.750	1.625	1.000	0.595	0.433	0.772	1.375	0.409	0.504	0.640
D1202	2.88	2.000	1.625	0.847	0.257	0.257	0.552	1.090	0.319	0.393	0.616
E1202	4.94	4.876	1.625	1.450	2.854	1.171	1.402	2.438	0.672	0.827	0.680
H1202	6.10	6.500	1.625	1.794	6.273	1.930	1.870	3.250	0.871	1.072	0.697
A1402	2.80	3.250	1.625	0.801	0.668	0.411	0.913	1.625	0.367	0.452	0.677
B1402	1.94	1.626	1.625	0.560	0.112	0.138	0.447	0.813	0.206	0.254	0.607

Table 2

Maximum pullout and slip loads for steel channel and channel nuts

Channel nuts size/thread	Channel all series	Pull out strength		Slip resistance			Torque
		lb	kN	lb	kN	Ft-lb	Nm
¼ – 20		600	2.7	300	1.3	6	8
⅜ – 18	A1200	800	3.6	500	2.2	11	15
	B1200						
⅝ – 16	C1200	1,000	4.4	800	3.6	19	25
	D1200	2,000	8.9	1,500	6.7	50	70
¾ – 11	E1200	2,500	11.1	1,500	6.7	100	135
	H1200	2,500	11.1	1,700	7.6	125	170
¼ – 20		600	2.7	300	1.3	6	8
⅜ – 18	A1400	800	3.6	400	1.8	11	15
	B1400	1,000	4.4	750	3.3	19	25
½ – 13		1,400	6.2	1,000	4.4	50	70

For aluminum channel the pull out load is calculated by multiplying the appropriate data by 50%. For slip resistance multiply by 75%.

Maximum pullout and slip loads for fiber glass channel and channel nuts

Channel nuts size/thread	Channel all series	Pull out strength		Slip resistance			Torque
		lb	kN	lb	kN	Ft-lb	Nm
¼ – 20	–	–	–	–	–	–	–
⅜ – 18	–	–	–	–	–	–	–
⅝ – 16	A1200	300	1.3	150	0.6	200	22.6
	D1200	300	1.3	150	0.6	200	22.6

Engineering data and specifications

Design data – metal framing channel

Table 3

Design loads for channel used as beam or column

Beam loads

Table 3 contains simple beam, uniformly distributed loads calculated at 25,000 psi material stress. Beam loads are based on channel being loaded across the X-X axis. Loads are also listed at reduced deflections for long spans.

Maximum loads at 25,000 psi stress

Maximum allowable deflections and maximum uniform loads for all spans at 25,000 psi material stress.

Reduced load for all $\frac{1}{180}$ span deflection

For moderate deflections on the longer spans, reduced loads are listed which will produce a deflection equal to $\frac{1}{180}$ of the span. When maximum loads do not induce deflections exceeding $\frac{1}{180}$ x the span length, reduced loads are not required.

Reduced load for $\frac{1}{360}$ span deflection

For very slight deflections on the longer spans, reduced loads are listed which will produce a deflection equal to $\frac{1}{360}$ of the span. When maximum loads do not induce deflections exceeding $\frac{1}{360}$ x the span length, reduced loads are not required.

Concentrated loads

To obtain values for concentrated loads from Table 3, multiply uniform load by 0.5 and deflection by 1.25.

Slotted, punched or KO channel

Reduce load rating by 5%.

Long span deep beams

Support in a manner to prevent rotation at supports and tie between supports to prevent twist.

Column loads

Allowable column loads given are for uniform axial loading with pinned ends. For eccentric loading or other end conditions reduce allowable loads according to standard engineering practice.

Dynamic loads

Allowable dynamic loads may be calculated by dividing the static loads shown in Table 3, by 2.08. Maximum beam and column loading for special materials is multiplied with the following factors:

Channel type	Beam type	Column load
Stainless steel	1	1
Aluminum	0.33	0.33

Warning

Load tables, charts and design criteria provided in this section are intended as guides only. Selection of proper product, installation intervals, erection and placement are the responsibility of the user.

ABB reserves the right to change material and finish specifications without notice, to improve its products.

Engineering data and specifications

Design data – metal framing channel

Table 3 (cont'd)

Single channel										Double channel											
Cat. no.	Depth (in.)	Ga.	Maximum uniform			$\frac{1}{160}$ Span		$\frac{1}{560}$ Span		Col. load	Cat. no.	Depth (in.)	Ga.	Maximum Uniform			$\frac{1}{160}$ Span		$\frac{1}{560}$ Span		Col. load
			Load	Defl.	Load	Defl.	Load	Defl.	Load					Defl.	Load	Defl.	Load	Defl.	Load	Defl.	
12 in. beam or column										12 in. beam or column											
A1200	1 $\frac{5}{8}$	12	3,534	0.014	-	0.067	-	0.033	10,533	A1202	3 $\frac{3}{4}$	12	-	0.008	-	0.067	-	0.033	21,177		
B1200	1 $\frac{3}{16}$	12	1,050	0.026	-	0.067	-	0.033	6,683	B1202	1 $\frac{5}{8}$	12	3,016	0.016	-	0.067	-	0.033	14,110		
C1200	1 $\frac{3}{8}$	12	2,584	0.016	-	0.067	-	0.033	9,345	C1202	2 $\frac{3}{4}$	12	-	0.010	-	0.067	-	0.033	18,990		
D1200	1	12	1,538	0.022	-	0.067	-	0.033	8,670	D1202	2	12	-	0.012	-	0.067	-	0.033	18,312		
E1200	2 $\frac{1}{16}$	12	6,650	0.010	-	0.067	-	0.033	13,830	E1202	4 $\frac{7}{8}$	12	-	0.005	-	0.067	-	0.033	27,623		
H1200	3 $\frac{3}{4}$	12	10,583	0.008	-	0.067	-	0.033	17,106	H1202	6 $\frac{1}{2}$	12	-	0.004	-	0.067	-	0.033	34,210		
A1400	1 $\frac{5}{8}$	14	2,434	0.015	-	0.067	-	0.033	7,575	A1402	3 $\frac{3}{4}$	14	-	0.008	-	0.067	-	0.033	15,250		
B1400	1 $\frac{3}{16}$	14	850	0.028	-	0.067	-	0.033	4,950	B1402	1 $\frac{5}{8}$	14	2,300	0.016	-	0.067	-	0.033	10,390		
18 in. beam or column										18 in. beam or column											
A1200	1 $\frac{5}{8}$	12	2,355	0.033	-	0.100	-	0.050	10,210	A1202	3 $\frac{3}{4}$	12	-	0.018	-	0.100	-	0.050	20,609		
B1200	1 $\frac{3}{16}$	12	700	0.059	-	0.100	-	0.050	6,058	B1202	1 $\frac{5}{8}$	12	2,011	0.036	-	0.100	-	0.050	13,440		
C1200	1 $\frac{3}{8}$	12	1,723	0.038	-	0.100	-	0.050	8,970	C1202	2 $\frac{3}{4}$	12	4,811	0.021	-	0.100	-	0.050	18,470		
D1200	1	12	1,025	0.052	-	0.100	-	0.050	7,930	D1202	2	12	-	0.028	-	0.100	-	0.050	17,942		
E1200	2 $\frac{1}{16}$	12	4,434	0.023	-	0.100	-	0.050	13,482	E1202	4 $\frac{7}{8}$	12	-	0.013	-	0.100	-	0.050	16,926		
H1200	3 $\frac{3}{4}$	12	7,055	0.016	-	0.100	-	0.050	16,693	H1202	6 $\frac{1}{2}$	12	-	0.009	-	0.100	-	0.050	33,390		
A1400	1 $\frac{5}{8}$	14	1,623	0.031	-	0.100	-	0.050	7,334	A1402	3 $\frac{3}{4}$	14	-	0.018	-	0.100	-	0.050	14,867		
B1400	1 $\frac{3}{16}$	14	566	0.063	-	0.100	453	0.050	4,150	B1402	1 $\frac{5}{8}$	14	1,534	0.036	-	0.100	-	0.050	9,910		
24 in. beam or column										24 in. beam or column											
A1200	1 $\frac{5}{8}$	12	1,766	0.058	-	0.133	-	0.067	9,842	A1202	3 $\frac{3}{4}$	12	4,858	0.031	-	0.133	-	0.067	19,974		
B1200	1 $\frac{3}{16}$	12	525	0.105	-	0.133	333	0.067	5,315	B1202	1 $\frac{5}{8}$	12	1,509	0.064	-	0.133	-	0.067	12,670		
C1200	1 $\frac{3}{8}$	12	1,291	0.066	-	0.133	-	0.067	8,545	C1202	2 $\frac{3}{4}$	12	3,609	0.038	-	0.133	-	0.067	17,890		
D1200	1	12	769	0.087	-	0.133	490	0.067	7,050	D1202	2	12	2,680	0.042	-	0.133	-	0.067	17,160		
E1200	2 $\frac{1}{16}$	12	3,325	0.039	-	0.133	-	0.067	13,082	E1202	4 $\frac{7}{8}$	12	-	0.021	-	0.133	-	0.067	26,143		
H1200	3 $\frac{3}{4}$	12	5,291	0.030	-	0.133	-	0.067	16,277	H1202	6 $\frac{1}{2}$	12	-	0.016	-	0.133	-	0.067	32,435		
A1400	1 $\frac{5}{8}$	14	1,216	0.056	-	0.133	-	0.067	7,058	A1402	3 $\frac{3}{4}$	14	3,425	0.033	-	0.133	-	0.067	14,426		
B1400	1 $\frac{3}{16}$	14	425	0.110	-	0.133	258	0.067	4,000	B1402	1 $\frac{5}{8}$	14	1,150	0.064	-	0.133	-	0.067	9,350		
30 in. beam or column										30 in. beam or column											
A1200	1 $\frac{5}{8}$	12	1,414	0.089	-	0.167	-	0.083	9,419	A1202	3 $\frac{3}{4}$	12	3,886	0.049	-	0.167	-	0.083	19,261		
B1200	1 $\frac{3}{16}$	12	420	0.164	-	0.167	266	0.083	4,465	B1202	1 $\frac{5}{8}$	12	1,206	0.100	-	0.167	-	0.083	11,803		
C1200	1 $\frac{3}{8}$	12	1,034	0.104	-	0.167	1,040	0.083	8,060	C1202	2 $\frac{3}{4}$	12	2,886	0.059	-	0.167	-	0.083	17,230		
D1200	1	12	0.615	0.129	-	0.167	389	0.083	6,650	D1202	2	12	2,128	0.084	-	0.167	-	0.083	16,480		
E1200	2 $\frac{1}{16}$	12	2,660	0.063	-	0.167	-	0.083	12,640	E1202	4 $\frac{7}{8}$	12	7,806	0.034	-	0.167	-	0.083	25,259		
H1200	3 $\frac{3}{4}$	12	4,234	0.046	-	0.167	-	0.083	15,698	H1202	6 $\frac{1}{2}$	12	-	0.025	-	0.167	-	0.083	31,395		
A1400	1 $\frac{5}{8}$	14	974	0.088	-	0.167	-	0.083	6,753	A1402	3 $\frac{3}{4}$	14	2,740	0.050	-	0.167	-	0.083	13,937		
B1400	1 $\frac{3}{16}$	14	340	0.172	-	0.167	165	0.083	3,420	B1402	1 $\frac{5}{8}$	14	920	0.100	-	0.167	-	0.083	8,730		

When no numbers are shown, use the maximum uniform load.
Deflections are given in inches; loads in lb

Engineering data and specifications

Design data – metal framing channel

Table 3 (cont'd)

Single channel										
Cat. no.	Depth (in.)	Ga.	Maximum uniform			$\frac{1}{800}$ Span		$\frac{1}{800}$ Span		Col. load
			Load	Defl.	Load	Defl.	Load	Defl.		
96 in. beam or column										
A1200	1 $\frac{5}{8}$	12	441	0.914	258	0.533	129	0.267	3,108	
B1200	1 $\frac{3}{16}$	12	-	-	42	0.533	21	0.267	-	
C1200	1 $\frac{3}{8}$	12	323	1.059	163	0.533	81	0.267	1,960	
D1200	1	12	192	1.400	998	0.533	49	0.267	-	
E1200	2 $\frac{7}{16}$	12	831	0.730	-	0.533	355	0.267	5,423	
H1200	3 $\frac{1}{4}$	12	1,323	0.478	-	0.533	739	0.267	7,059	
A1400	1 $\frac{5}{8}$	14	304	0.903	180	0.533	90	0.267	2,615	
B1400	1 $\frac{3}{16}$	14	-	-	32	0.533	16	0.267	-	
108 in. beam or column										
A1200	1 $\frac{5}{8}$	12	393	1.156	204	0.600	102	0.300	2,456	
B1200	1 $\frac{3}{16}$	12	-	-	33	0.600	16	0.300	-	
C1200	1 $\frac{3}{8}$	12	288	1.350	128	0.600	64	0.300	-	
D1200	1	12	171	1.783	76	0.600	38	0.300	-	
E1200	2 $\frac{7}{16}$	12	739	0.790	561	0.600	281	0.300	4,291	
H1200	3 $\frac{1}{4}$	12	1,176	0.605	-	0.600	584	0.300	5,579	
A1400	1 $\frac{5}{8}$	14	270	1.141	142	0.600	71	0.300	1,708	
B1400	1 $\frac{3}{16}$	14	-	-	25	0.600	13	0.300	-	
120 in. beam or column										
A1200	1 $\frac{5}{8}$	12	354	1.425	165	0.667	83	0.333	-	
B1200	1 $\frac{3}{16}$	12	-	-	27	0.667	13	0.333	-	
C1200	1 $\frac{3}{8}$	12	259	1.663	104	0.667	52	0.333	-	
D1200	1	12	154	2.202	62	0.667	31	0.333	-	
E1200	2 $\frac{7}{16}$	12	665	0.976	455	0.667	227	0.333	3,478	
H1200	3 $\frac{1}{4}$	12	1,059	0.746	-	0.667	473	0.333	4,521	
A1400	1 $\frac{5}{8}$	14	244	1.413	114	0.667	57	0.333	-	
B1400	1 $\frac{3}{16}$	14	-	-	21	0.667	10	0.333	-	
144 in. beam or column										
A1200	1 $\frac{5}{8}$	12	-	-	115	0.800	57	0.400	-	
-	-	-	-	-	-	-	-	-	-	
C1200	1 $\frac{3}{8}$	12	-	-	72	0.800	36	0.400	-	
E1200	2 $\frac{7}{16}$	12	554	1.400	315	0.800	158	0.400	-	
H1200	3 $\frac{1}{4}$	12	883	1.075	657	0.800	328	0.400	-	
A1400	1 $\frac{5}{8}$	14	-	-	80	0.800	40	0.400	-	

When no numbers are shown, use the maximum uniform load.
Deflections are given in inches; loads in lb

Double channel										
Cat. no.	Depth (in.)	Ga.	Maximum uniform			$\frac{1}{800}$ Span		$\frac{1}{800}$ Span		Col. load
			Load	Defl.	Load	Defl.	Load	Defl.		
96 in. beam or column										
A1202	3 $\frac{1}{4}$	12	1,215	0.509	-	0.533	637	0.267	7,675	
B1202	1 $\frac{5}{8}$	12	378	1.019	197	0.533	99	0.267	-	
C1202	2 $\frac{3}{4}$	12	903	0.603	-	0.533	400	0.267	6,640	
D1202	2	12	535	0.813	263	0.533	176	0.267	2,942	
E1202	4 $\frac{7}{8}$	12	2,440	0.340	-	0.533	1,917	0.267	10,875	
H1202	6 $\frac{1}{2}$	12	4,021	0.255	-	0.533	-	0.267	14,120	
A1402	3 $\frac{1}{4}$	14	856	0.509	-	0.533	449	0.267	5,951	
B1402	1 $\frac{5}{8}$	14	288	1.020	150	0.533	75	0.267	-	
108 in. beam or column										
A1202	3 $\frac{1}{4}$	12	1,080	0.644	-	0.600	503	0.300	6,071	
B1202	1 $\frac{5}{8}$	12	355	1.290	156	0.600	78	0.300	-	
C1202	2 $\frac{3}{4}$	12	801	0.763	632	0.600	316	0.300	5,250	
D1202	2	12	476	1.029	208	0.600	139	0.300	2,324	
E1202	4 $\frac{7}{8}$	12	2,169	0.430	-	0.600	1,515	0.300	8,599	
H1202	6 $\frac{1}{2}$	12	3,574	0.323	-	0.600	-	0.300	11,160	
A1402	3 $\frac{1}{4}$	14	761	0.644	-	0.600	355	0.300	4,702	
B1402	1 $\frac{5}{8}$	14	255	1.290	119	0.600	59	0.300	-	
120 in. beam or column										
A1202	3 $\frac{1}{4}$	12	971	0.795	-	0.667	408	0.333	-	
B1202	1 $\frac{5}{8}$	12	301	1.588	126	0.667	63	0.333	-	
C1202	2 $\frac{3}{4}$	12	721	0.941	512	0.667	256	0.333	4,250	
D1202	2	12	428	1.271	168	0.667	112	0.333	1,883	
E1202	4 $\frac{7}{8}$	12	1,951	0.531	-	0.667	1,227	0.333	6,946	
H1202	6 $\frac{1}{2}$	12	3,216	0.398	-	0.667	-	0.333	9,040	
A1402	3 $\frac{1}{4}$	14	685	0.796	-	0.667	287	0.333	3,805	
B1402	1 $\frac{5}{8}$	14	230	1.600	96	0.667	48	0.333	-	
144 in. beam or column										
A1202	3 $\frac{1}{4}$	12	810	1.145	566	0.800	283	0.400	-	
B1202	1 $\frac{5}{8}$	12	-	-	88	0.800	44	0.400	-	
C1202	2 $\frac{3}{4}$	12	601	1.350	355	0.800	178	0.400	-	
E1202	4 $\frac{7}{8}$	12	1,626	0.764	-	0.800	852	0.400	-	
H1202	6 $\frac{1}{2}$	12	2,680	0.573	-	0.800	1,873	0.400	-	
A1402	3 $\frac{1}{4}$	14	571	1.146	399	0.800	199	0.400	-	
B1402	1 $\frac{5}{8}$	14	-	-	67	0.800	33	0.400	-	

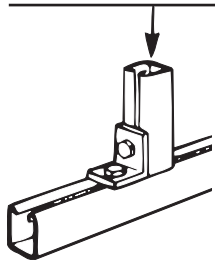
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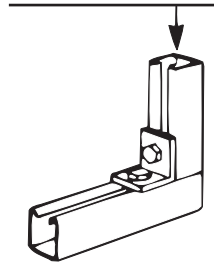
Table 4

Safe bearing loads for 1½ in. channel and combinations.

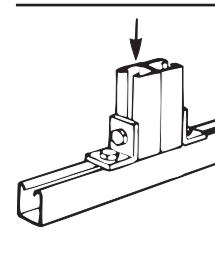
Safety factor of 2½



Section	Recommended load in lb
A1200	5,000
A1400	3,500
B1200	6,000
B1400	3,400
C1200	5,000
E1200	5,000
H1200	4,000



Section	Recommended load in lb
A1200	3,500
A1400	2,500
B1200	4,000
B1400	2,600
C1200	3,500
E1200	3,500
H1200	2,000



Section	Recommended load in lb
A1200	8,000
A1400	5,500
B1200	9,000
B1400	4,800
C1200	8,000
E1200	8,000
H1200	5,500

Table 5

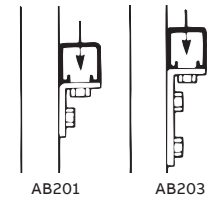
Design load table for typical channel connections.

Safety factor of 2½ based on ultimate strength of the connection.

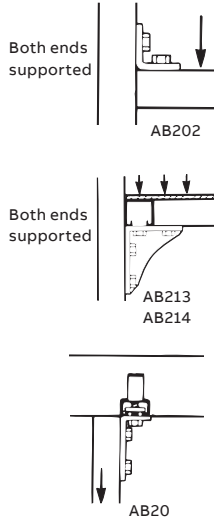
Load diagrams indicate up to three design loads, for 12 gauge and 14 gauge channel applications.

90° Fittings (when used in position shown)

Cat. no.	Section	Recommended load in lb
AB202	A1200	1,500
	A1400	1,000
AB203	A1200	2,000
	A1400	1,500
AB201		700
AB203		700




Cat. no.	Section	Recommended load in lb
AB202	A1200	1,000
	A1400	650
AB213	A1200	3,000
AB214	A1400	2,000
AB20		1,500



Cat. no.	Section	Recommended load in lb
AB205	A1200	2,000
AB216	A1400	2,000
AB204	A1200	1,500
AB215	A1400	1,000

Flat plate fittings

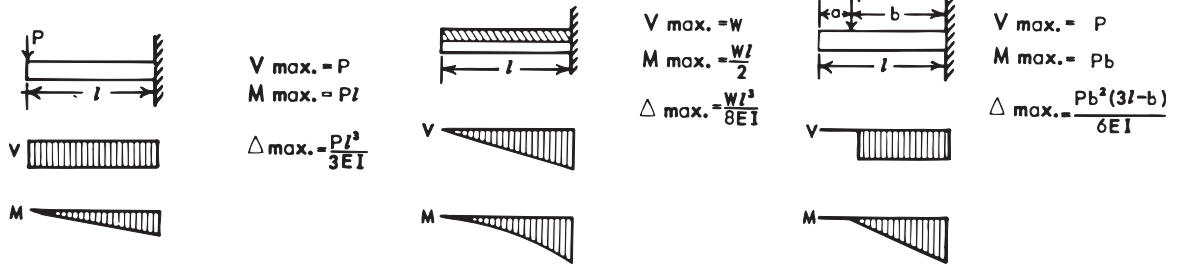
Cat. no.	Section	Recommended load in lb
AB206	A1200	1,000
	A1400	800



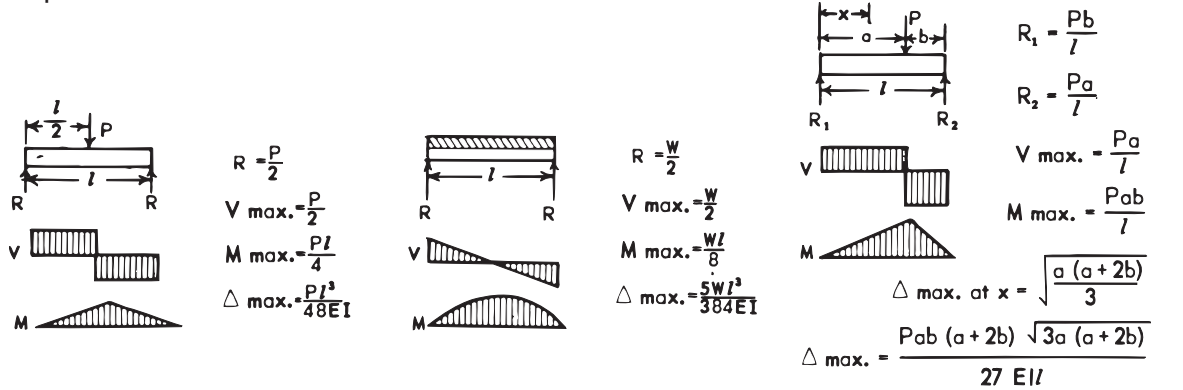
Engineering data and specifications

Design applications

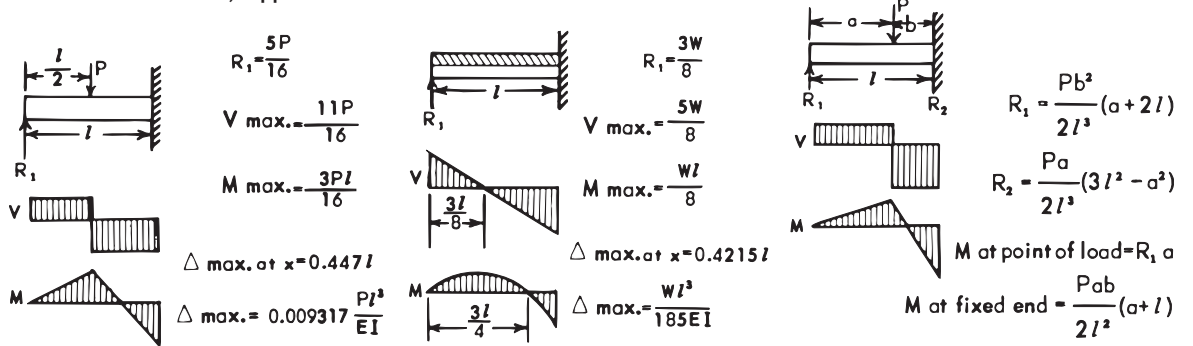
Cantilever beams



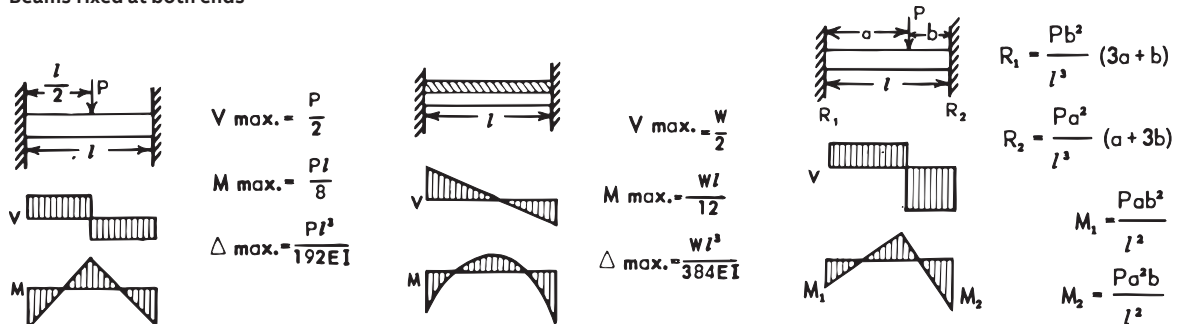
Simple beams



Beams fixed on one end, supported at the other end



Beams fixed at both ends



R - Reaction
 M - Moment
 P - Concentrated load

W - Total uniform load
 V - Shear

Δ - Deflection
 E - Modulus of Elasticity
 I - Moment of Inertia

Engineering data and specifications

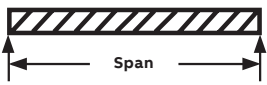



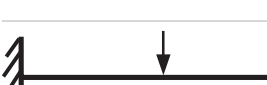

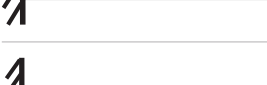




Design applications

Table 6

Conversion factors for beams with various static loading conditions

Load tables on pages A68 through A72 for A, B, C, E, and H series channel are for single span beams supported at the ends. These can be used in the majority of cases. There are

times when it is necessary to know what happens with other loading and support conditions. Some common arrangements are shown in Table 6. Simply multiply the loads from the design load tables times the factors given in Table 6.

	Load and support condition	Load factor	Deflection factor
	1. Simple beam – Uniform load	1.00	1.00
	2. Simple beam – Concentrated load at center	0.50	1.25
	3. Simple beam – Two equal concentrated loads at 1/4 points	1.00	1.10
	4. Beam fixed at both ends – Uniform load	1.50	0.30
	5. Beam fixed at both ends – Concentrated load at center	1.00	0.40
	6. Cantilever beam – Uniform load	0.25	2.40
	7. Cantilever beam – Concentrated load at end	0.12	3.20
	8. Continuous beam – Two equal spans – Uniform load on one span	1.30	0.92
	9. Continuous beam – Two equal spans – Uniform load on both ends	1.00	0.42
	10. Continuous beam – Two equal spans – Concentrated load at center of one span	0.62	0.71
	11. Continuous beam – Two Equal Spans – Concentrated load at center of both spans	0.67	0.48

Engineering data and specifications

Design applications

Example I

Problem:

Determine the load and deflection of an A1200 beam continuous over one support and loaded uniformly on one span.

Solution:

- A. From load table 3 for A1200 the load for a 5 ft. 0 in. span is 706 lb and deflection is 0.358 in.
- B. Multiply by factors from Table 6.
 Load = 706 lb x 1.30 = 917.8 lb
 Deflection = 0.358 in. x 0.92 = 0.329 in.



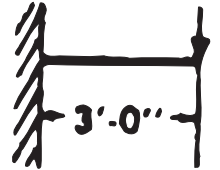
Example II

Problem:

Determine load and deflection of an E1200 cantilever beam with a concentrated load on the end.

Solution:

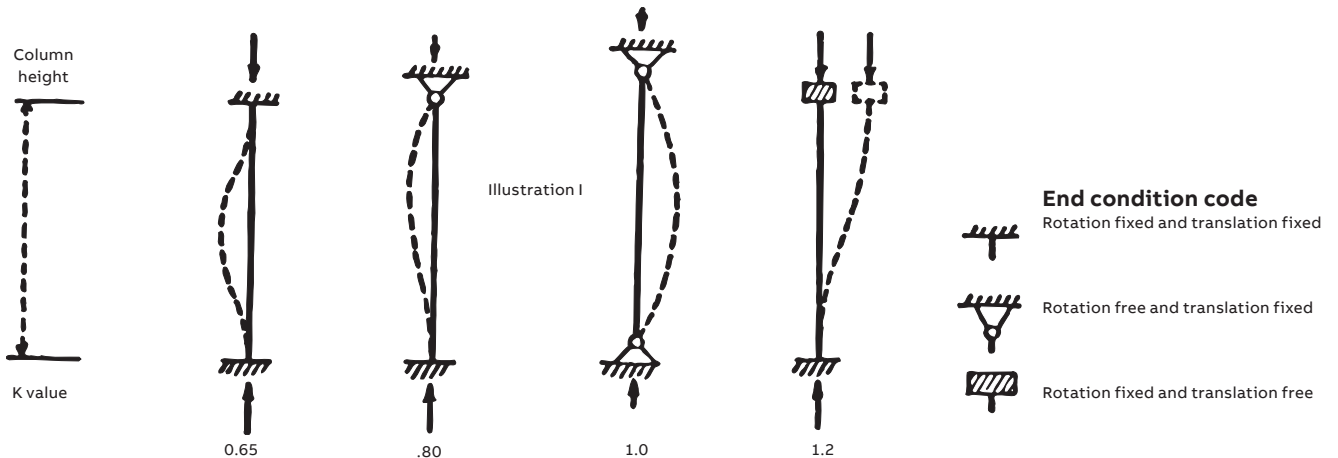
- A. From load table 3 for E1200 the load for a 3 ft. 0 in. span is 2,216 lb and deflection is 0.088 in.
- B. Multiply by factors from Table 6.
 Load = 2,216 lb x 0.12 = 265.9 lb
 Deflection = 0.088 in. x 3.20 = 0.282 in.



Column loading

The load bearing capacity of column or compression members is a function of the inherent configurational strength, the unbraced length and design of the end connections.

Values of axial column loading given in Table 3 were calculated using a rotationally free and translation fixed correction at each end (see Illustration I). This gives an end condition constant (K) of 1.



If other end conditions are used, axial loading should be calculated using procedures in the AISI specification for the design of cold formed steel structural members (SG671) and the engineering values for Superstrut channel given in Table 1.

Table 7

Load carrying capacities of hot-rolled steel rod

Nominal rod dia. (in.)	Root area thread (in.)	Design load lb for serv. temperature	
		343 °C (650 °F)	399 °C (750 °F)
3/8	0.068	610	540
1/2	0.126	1,130	1,010
5/8	0.202	1,810	1,610
3/4	0.302	2,710	2,420
7/8	0.419	3,770	3,360

Safety factor of 5.

Table 8

Rod size determined by pipe size for fire protection

Pipe size (in.)	Rod size (in.)
3/4 to 2	3/8
2 1/2 to 3 1/2	1/2
4 to 5	5/8
6	3/4
8 to 12	7/8

Engineering data and specifications

Design applications

Table 9

Maximum spacing between pipe supports

Steel pipe																		
Nom. pipe size (in.)	½	¾	1	1½	2	2½	3	3½	4	5	6	8	10	12	14	16	18	20
Max. spacing (ft.)	5	6	7	9	10	11	12	13	14	16	17	19	22	23	25	27	28	30
Copper pipe																		
Nom. pipe size (in.)	½	¾	1	1¼	1½	2	2½	3	3½	4								
Max. spacing (ft.)	5	6	6	7	8	9	10	10	11	12								

Table 10

Minimum spacing (inches) between centers of standard pipe when using Superstrut #702 pipe straps

Nom. pipe size (in.)	½	¾	1	1¼	1½	2	2½	3	3½	4	5	6	8
½	1⅜	—	—	—	—	—	—	—	—	—	—	—	—
¾	1⅝	1⅞	—	—	—	—	—	—	—	—	—	—	—
1	1½	1⅝	1¾	—	—	—	—	—	—	—	—	—	—
1¼	1¾	1⅞	2	2¼	—	—	—	—	—	—	—	—	—
1½	1⅝	2⅜	2⅝	2⅞	2⅞	—	—	—	—	—	—	—	—
2	2⅜	2⅝	2½	2¾	2⅞	3⅞	—	—	—	—	—	—	—
2½	2⅞	2⅝	2¾	3	3⅞	3⅞	3⅝	—	—	—	—	—	—
3	2⅜	2⅝	3⅜	3⅝	3⅞	3¾	4	4⅝	—	—	—	—	—
3½	3⅞	3¾	3⅞	3⅝	3¾	4⅜	4⅝	4⅝	4⅝	—	—	—	—
4	3⅞	3⅝	3⅝	4⅜	4⅞	4⅝	4⅝	5¼	5⅝	—	—	—	—
6	4¾	4⅞	5	5¼	5⅞	5⅝	5⅞	6⅜	6½	6⅜	7⅞	8⅞	—
8	5⅞	6	6⅞	6⅞	6½	6¾	7	7⅞	7⅞	8	8⅞	9¼	10⅞

Engineering data and specifications

Design applications

Table 11

Standard dimensions and weights of piping materials and conduit

Mechanical (ANSI & API standard, Schedule 40)					
Nominal std. pipe size (in.)	Pipe O.D. (in.)	Coupling O.D. (in.)	Weight of pipe lb/ft.	Weight of pipe filled w/water lb/ft.	
½	.84	1.06	0.85	0.98	
¾	1.05	1.31	1.13	1.36	
1	1.32	1.58	1.68	2.05	
1¼	1.66	1.90	2.27	2.92	
1½	1.90	2.20	2.72	3.60	
2	2.38	2.75	3.65	5.11	
2½	2.88	3.25	5.79	7.87	
3	3.50	4.00	7.58	10.78	
3½	4.00	4.63	9.11	13.39	
4	4.50	5.00	10.79	16.30	
5	5.56	6.30	14.62	23.28	
6	6.63	7.39	18.97	31.48	
8	8.63	9.23	28.56	50.24	
10	10.75	–	41.00	74.00	
12	12.75	–	50.00	99.00	
14	14.00	–	64.00	122.00	
16	16.00	–	63.00	142.00	
18	18.00	–	71.00	172.00	
20	20.00	–	79.00	205.00	
22	22.00	–	87.00	240.00	
24	24.00	–	95.00	277.00	
26	26.00	–	103.00	322.00	
28	28.00	–	111.00	364.00	
30	30.00	–	119.00	410.00	

Electrical conduit					
Nominal conduit size (in.)	Conduit O.D. (in.)	Weight of conduit lb/ft.	Rigid steel		Thin wall (EMT)
			Weight of conduit w/non-lead covered conductor lb/ft.		Conduit O.D. (in.)
½	0.84	0.85	1.04	0.71	0.29
¾	1.05	1.13	1.40	0.92	0.44
1	1.32	1.68	2.35	1.16	0.64
1¼	1.66	2.28	3.58	1.51	0.95
2	2.38	3.68	7.21	2.20	1.40
2½	2.88	5.82	10.22	2.88	2.30
3	3.50	7.62	14.51	3.50	2.70
4	4.50	10.89	21.48	4.50	4.00

Includes weight of heaviest conductor combination.

Engineering data and specifications

Design applications

Table 12

Extra strong pipe (ANSI & API standard, Schedule 80)

A.S.A. B36.10 Schedule nos. and nominal wall thickness designations						
Nominal pipe size (in.)	O.D. (in.)	Wall thickness (in.)	I.D. (in.)	Weight of pipe lb/ft.	Water weight per ft. of pipe lb	Weight of pipe filled w/Water lb/ft.
Extra strong pipe and Schedule 80 pipe (through 8 in.)						
3/8	0.675	0.126	0.423	0.74	0.061	0.801
1/2	0.840	0.147	0.546	1.09	0.101	1.191
3/4	1.050	0.154	0.742	1.47	0.188	1.668
1	1.315	0.179	0.957	2.17	0.311	2.481
1 1/4	1.660	0.191	1.278	3.00	0.555	3.555
1 1/2	1.900	0.200	1.500	3.63	0.765	4.395
2	2.375	0.218	1.939	5.03	1.279	6.309
2 1/2	2.875	0.276	2.323	7.66	1.834	9.497
3	3.500	0.300	2.900	10.30	2.860	13.16
3 1/2	4.000	0.318	3.364	12.55	3.850	16.35
4	4.500	0.337	3.826	15.00	4.98	19.98
5	5.563	0.375	4.813	20.80	7.89	28.69
6	6.625	0.432	5.761	28.60	11.29	39.89
8	8.625	0.500	7.625	43.40	19.79	63.20
Extra strong pipe (10 in. through 24 in. OD)						
10	10.750	0.500	9.750	54.70	32.30	87.00
12	12.750	0.500	11.750	65.40	47.00	112.40
14 OD	14.000	0.500	13.000	72.10	57.50	129.60
16 OD	16.000	0.500	15.000	82.80	76.50	159.30
18 OD	18.000	0.500	17.000	93.50	98.40	191.90
20 OD	20.000	0.500	19.000	104.10	122.80	226.90
24 OD	24.000	0.500	23.000	125.50	180.10	305.60
Schedule 80 pipe (10 in. through 24 in. OD)						
10	10.750	0.593	9.564	64.300	31.10	95.40
12	12.750	0.687	11.376	88.50	44.00	132.50
14 OD	14.000	0.750	12.500	106.10	53.20	159.30
16 OD	16.000	0.842	14.314	136.50	69.70	206.20
18 OD	18.000	0.937	16.126	170.80	88.50	259.30
20 OD	20.000	1.031	17.938	208.90	109.40	318.30
24 OD	24.000	1.218	21.564	296.40	158.30	454.70

Engineering data and specifications

Design applications

Table 13

Pipe covering weights (thickness intended as guide, only)

Nominal pipe size (in.)	260°		360°		440°		525°		600°		700°		800°	
	Thick. (in.)	lb/ft.	Thick. (in.)	lb/ft.	Thick. (in.)	lb/ft.	Thick. (in.)	lb/ft.	Thick. (in.)	lb/ft.	Thick. (in.)	lb/ft.	Thick. (in.)	lb/ft.
1	1	0.68	1	0.68	1	0.68	1	0.68	1½	1.19	1½	1.19	1½	1.19
1¼	1	0.75	1	0.75	1	0.75	1	0.75	1½	1.27	1½	1.27	2	1.82
1½	1	0.88	1	0.88	1	0.88	1	0.88	1½	1.45	1½	1.45	2	1.87
2	1	1.01	1	1.01	1	1.01	1½	1.53	1½	1.53	2	2.50	2	2.50
2½	1	1.15	1	1.15	1	1.15	1½	1.69	1½	1.69	2	2.50	2½	3.22
3	1	1.28	1	1.28	1	1.28	1½	2.09	1½	2.09	2	2.98	2½	3.98
3½	1	1.44	1	1.44	1½	2.29	1½	2.29	2	3.00	2	3.12	2½	4.30
4	1	1.60	1	1.60	1½	2.49	1½	2.49	2	3.49	2	3.49	2½	4.62
5	1	1.84	1	1.84	1½	2.84	1½	2.84	2	3.97	2	3.97	2½	5.92
6	1½	3.13	1½	3.13	1½	3.13	1½	3.13	2	4.54	2	4.54	2½	6.75
8	1½	4.06	1½	4.06	1½	4.06	1½	4.06	2	5.56	2	5.56	2½	7.61

Thickness and weight of calcium silicate covering.

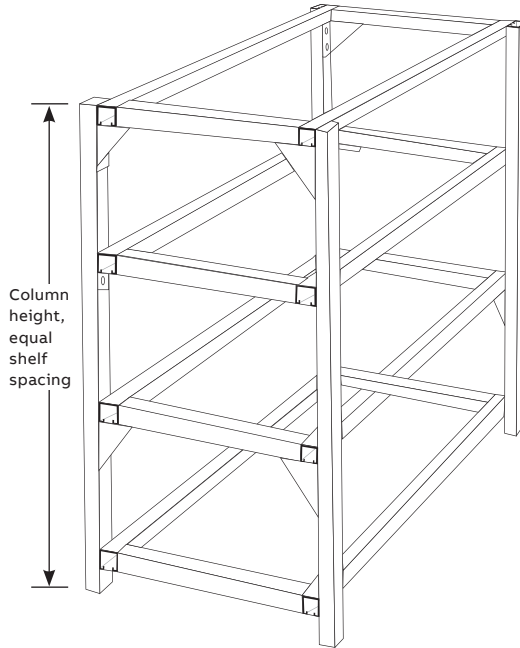
Engineering data and specifications

Design applications – mechanical support

Table 14

Column loading for rack construction

Typical general storage rack for use with plywood or other decking.



General storage racks

- Pallet racks**
- Barrel racks**
- Bulk furniture racks**
- Cable racks**
- Bar stock racks**
- Display racks**
- Special purpose racks**

For uniform loads on horizontal members, see Table 3

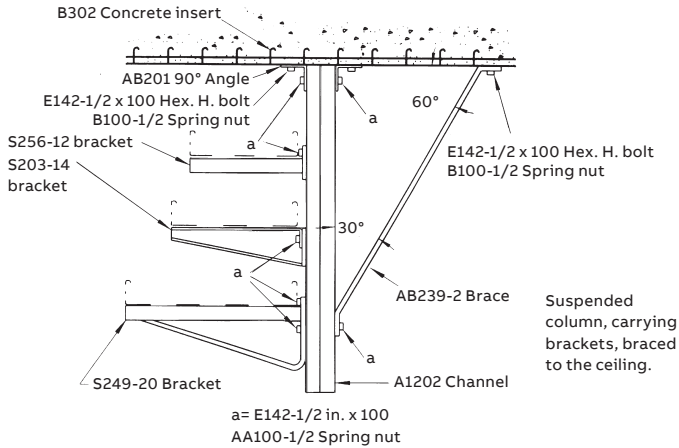
Allowable load in lb per upright

Column height	Cat. no.	Number of shelves per upright								
		2	3	4	5	6	7	8	9	10
6'	A1200	2,237	1,925	1,650	1,437	1,290	-	-	-	-
	A1202	4,170	3,580	3,100	2,730	2,450	-	-	-	-
	B1400	800	820	790	700	630	-	-	-	-
	B1402	1,930	1,700	1,500	1,300	1,190	-	-	-	-
7'	A1200	2,150	1,850	1,630	1,425	1,280	1,150	-	-	-
	A1202	4,000	3,525	3,000	2,700	2,430	2,200	-	-	-
	B1400	650	790	760	685	615	550	-	-	-
	B1402	1,800	1,650	1,450	1,300	1,180	750	-	-	-
8'	A1200	2,000	1,820	1,600	1,400	1,250	1,150	1,050	-	-
	A1202	3,900	3,475	3,000	2,700	2,400	2,185	2,000	-	-
	B1400	580	750	730	660	610	540	510	-	-
	B1402	1,650	1,610	1,450	1,300	1,160	940	970	-	-
9'	A1200	1,950	1,780	1,575	1,400	1,250	1,130	1,030	950	-
	A1202	3,800	3,400	3,020	2,675	2,400	2,180	1,975	1,800	-
	B1400		600	665	600	580	540	500	475	-
	B1402	1,500	1,500	1,430	1,275	1,160	1,000	900	800	-
10'	A1200	1,870	1,700	1,500	1,300	1,200	1,100	1,000	900	800
	A1202	3,600	3,300	3,000	2,650	2,350	2,000	1,975	1,800	1,650
	B1400		550	650	625	580	535	490	450	425
	B1402	1,450	1,480	1,400	1,250	1,140	1,040	960	885	825

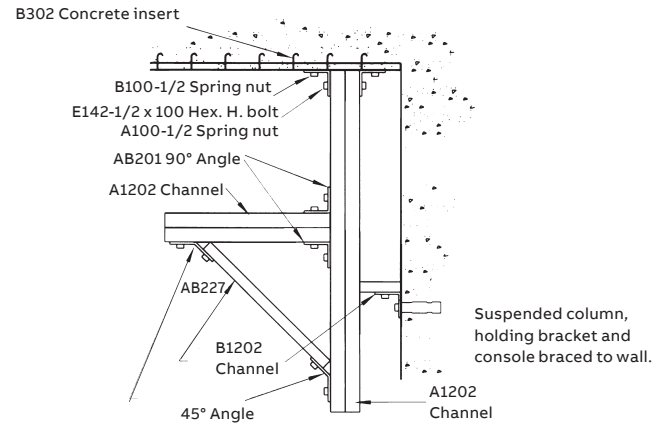
Engineering data and specifications

Design applications – mechanical support

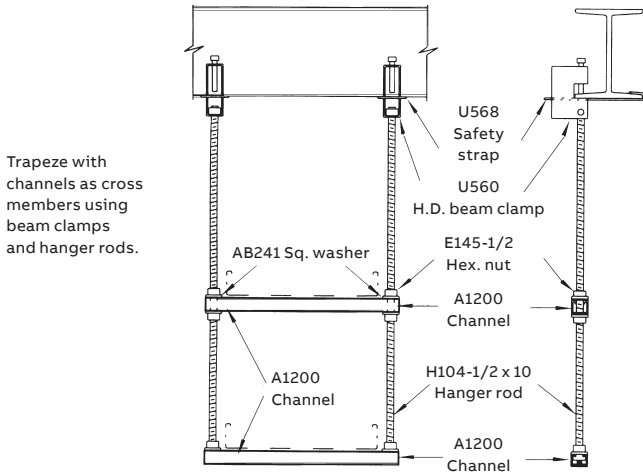
Example 1



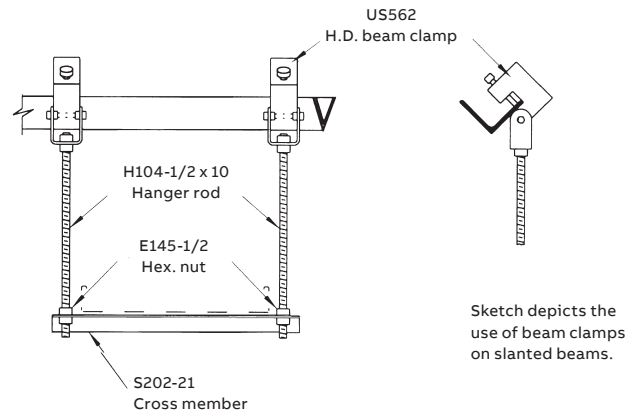
Example 2



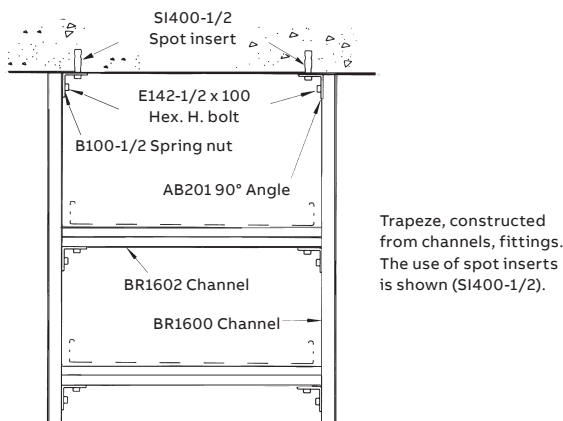
Example 3



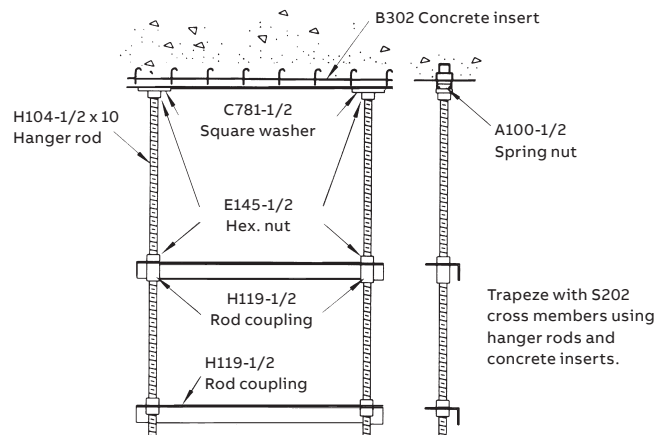
Example 4



Example 5



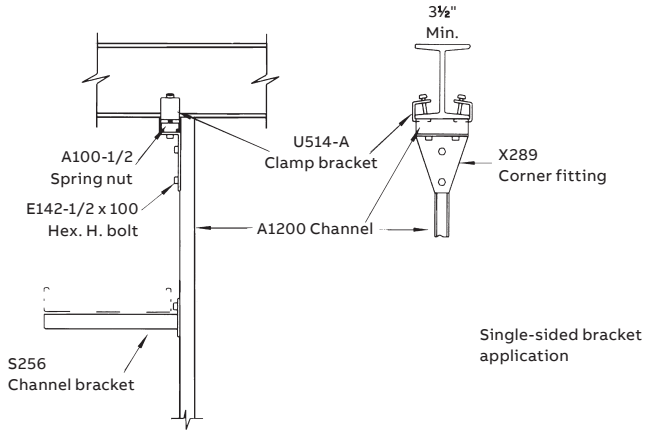
Example 6



Engineering data and specifications

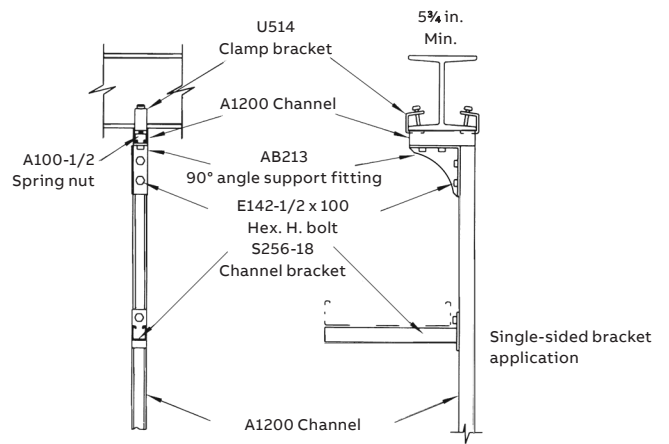
Design applications – mechanical support

Example 7

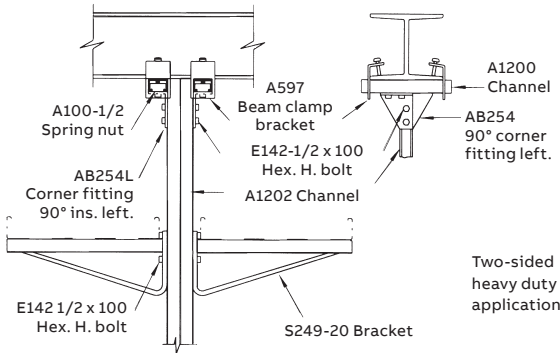


*Note: Brace should be used for lengths greater than 30 in.

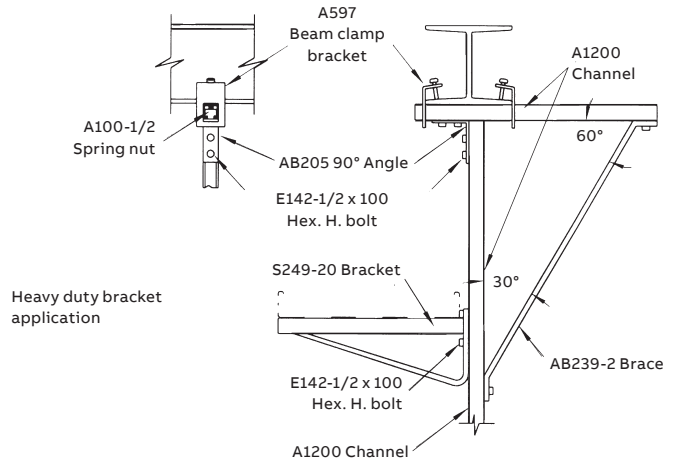
Example 8



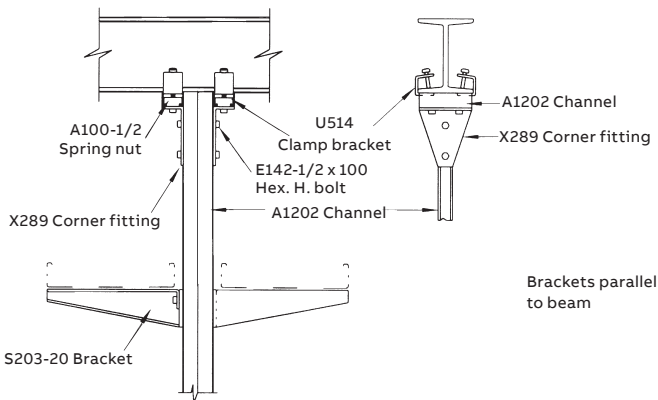
Example 9



Example 10



Example 11



Example 12

